EXHIBIT B

COMPUTER STRUCTURAL ANALYSIS
& ENGINEERING EVALUATION
OF THE
599 FT. (706 FT. OVERALL) GUYED TOWER
CATONSVILLE, MARYLAND

FINAL REVISION

JUNE 1993

FOR

SCRIPPS HOWARD BROADCASTING COMPANY

BY

VLISSIDES ENTERPRISES, INC.

7601 BURFORD DRIVE

McLEAN, VIRGINIA 22102

(703) 442-8320

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SECTION A

INTRODUCTION

The subject structure is assumed to be a 599 ft. (706 ft. overall) guyed tower located in Catonsville, Maryland (Coordinates: 39^{o} 17' 13"; 76^{o} 45' 16"). The tower has a triangular cross-section with a face width of 4 ft. It is supported on a hinged base with seven guy levels of three guys each. The tower was designed and manufactured by Utility Tower Company in 1969.

The purpose of this analysis is to investigate the structural capability of the tower to support the Channel 2 TV antenna on top and its two or one 3-1/8" transmission lines, in addition to the existing antennas and transmission lines.

The following assumptions have been made regarding the major charactetistics of the structural system employed in the design of the subject tower:

a) The tower is assumed shortened to 599 ft. in order to accommodate a 104 ft. Alan Dick Channel 2 Antenna and a 3 ft. lighting beacon.

b) Section panels were assumed to be approximately 5 ft. in height.

c) The tower span lengths were estimated to be 93.5 ft., 95.2 ft., 95.2 ft., 95.2 ft., 94.5 ft., 95.2 ft. and 29.8 ft., for spans #1 through #7 respectively.

d) The inner and outer guy anchors were estimated to

e) The guy cables are E.H.S. cables with estimated diameters of 5/8", 5/8", 3/4", 5/8", 3/4", 7/8" and 1" for guy levels #1 through #7 respectively.

f) The tower legs were assumed to be of 3.5" O.D. with 0.300" wall thickness in the bottom 500 ft. of the tower and 0.216" wall thickness from 500 ft. to top.

g) All the diagonal members were assumed to be solid rods of 5/8" diameter.

h) All the horizontal girts were assumed to be solid rods of 1" diameter.

i) All the tower members were assumed made of 50,000 psi minimum yield strength steel.

j) The tower sections are of all welded construction and are bolted together through round splice plates on each leg.

k) The tower color banding, after shortening of the tower, will not be in accordance with the FAA Advisory Circular 70/7460-1H for towers under 700 ft. height.

The overall structural system of the tower resists the guy reactions, the wind loads and bending moments by having the legs in tension or compression; the diagonals in tension; and the girts in compression. The structural integrity of the tower depends mainly on the buckling load capacity of the legs and girts and the tension load capacity of the diagonals and guy cables.

The subject tower was analyzed under a 75 mph basic wind velocity (no ice) in accordance with the EIA/TIA Standard 222-E. The computed wind pressure was applied to all tower members, antennas and ancillary items (transmission lines, ladder, conduits, etc. No ice loading was considered in this analysis.

ORGANIZATION OF ANALYSIS

1. The following rigorous computer analysis was performed [redacted]

Whip	363 ft.	7/8" Heliax
Whip	375 ft.	7/8" Heliax
Whip	393 ft.	7/8" Heliax
Whip	402 ft.	7/8" Heliax
Whip	403 ft.	7/8" Heliax
Whip	486 ft.	7/8" Heliax
Whip	501 ft.	7/8" Heliax
Whip	511 ft.	7/8" Heliax
Whip	523 ft.	7/8" Heliax
Whip	537 ft.	7/8" Heliax
Long Whip	549 ft.	1-5/8" Heliax
2-Bay FM	573 ft.	3" Heliax
Whip	587 ft.	7/8" Heliax
	Top	1" Conduit
Alan Dick Superturnstile Channel 2	Top	(2) 3-1/8" Rigid Coax

The type, size, location and number of antennas and transmission lines were taken from sketch of tower prepared by Gerhold, Cross & Etzel, Professional Land Surveyors, Dated 1/20/92. The type of Channel 2 antenna and its transmission lines were assumed. The transmission lines have been taken as are on the tower without any bundling.

c) Case 3(a) Tower in its assumed configuration, 599 ft. (706 overall), under a 75 mph basic wind velocity and no ice, in accordance with EIA/TIA Standard 222-E specifications and the following antenna and transmission line loadings:

Antenna	Elev. (Ft.)	Transmission Line
Yagi	29 ft.	7/8" Heliax
Whip	98 ft.	7/8" Heliax

Whip	119 ft.	7/8" Heliax
3-Bay Communication	180 ft.	1-5/8" Heliax
8-Element	190 ft.	1-5/8" Heliax
4' Dish w/Radome	230 ft.	1-5/8" Heliax
Whip	289 ft.	7/8" Heliax
Whip	363 ft.	7/8" Heliax
Whip	375 ft.	7/8" Heliax
Whip	393 ft.	7/8" Heliax
Whip	402 ft.	7/8" Heliax
Whip	403 ft.	7/8" Heliax
Whip	486 ft.	7/8" Heliax
Whip	501 ft.	7/8" Heliax
Whip	511 ft.	7/8" Heliax
Whip	523 ft.	7/8" Heliax
Whip	537 ft.	7/8" Heliax
Long Whip	549 ft.	1-5/8" Heliax
2-Bay FM	573 ft.	3" Heliax
Whip	587 ft.	7/8" Heliax
	Top	1" Conduit
Alan Dick Superturnstile Channel 2	Top	(1) 3-1/8" Rigid Coax

The type, size, location and number of antennas were taken from sketch of tower prepared by Gerhold, Cross & Etzel, Professional Land Surveyors, Dated 1/20/92. The existing transmission lines sizes and types were assumed. All the assumed 7/8" and 1-5/8" Heliax transmission lines were considered in three bundles. The type of Channel 2 antenna and its transmission line were assumed.

d) Case 4(a) Same as in Case 3 (a) above, except all the assumed 7/8" and 1-5/8" Heliax transmission lines were considered in one bundle up the tower.

2. For all computer runs the results are given as follows

existing antenna and transmission line loads.

b) <u>Allowable Guy Cable Safety Factors:</u> For towers less than 700 ft. in height, in accordance with EIA/TIA Standard 222-E, the guy cable minimum safety factor requirement is 2.00

c) <u>Tower Design Assumptions:</u> All of my assumptions regarding the characteristics of the tower structural system are based on exhaustive study of the structure through personal observations with the use of high power binoculars, high power surveying instruments, large number of photographs taken from short distance with high power lenses, thirty years of experience in dealing with thousands of communications towers' design, analysis, fabrication, installation, inspection and overall construction, and finally, knowledge of the tower designs of the Utility Tower Company. In making my assumptions concerning the characteristics of the tower structural system, I was very careful in giving the opposition every possible advantage.

d) <u>Type of Structural Steel Assumed:</u> I assumed that all structural members on the tower (tower legs, horizontals and diagonals) are made of 50,000 psi high-strength steel, which is very questionable. It is more probable that the steel used for the tower legs is 35,000 psi ASTM A53 pipe and for the diagonals and horizontals ASTM A36 solid bars, which would make the results of the Analysis of the Tower much worse.

e) <u>Tower Antennae Loads:</u> Examining the tower photographs presented in my tower analysis report, it is obvious that at the top of the tower is the skeleton of a ten bay FM antenna without radiating elements or with very small radiating elements. Because I was not very sure about the type of antenna, I totally disregarded this significant antenna load and I did not include it in the tower analysis. All other antennas on the tower were included in the Tower Analysis.

f) <u>Tower Geometry:</u> The geometry of the tower was carefully measured through surveying instruments and the panel height, type and diameter of the tower leg was verified during these optical measurements.

g) <u>Transmission Lines:</u> Twenty-two transmission lines total are used to feed the various indicated anten- [illegible]

h) Type and Location of Antennas and Transmission Lines: I located the antenna elevations and transmission lines from direct observations and photographs and I verified the antenna elevations by using the land surveyor's report.

i) Ice Loading on the Tower: The tower geographical area is subject to icing conditions, with 0.5 inch radial glaze ice loadings being quite possible. EIA/TIC Standard 222-E leaves the ice loading decision up to the structural tower engineer. Again, being consistent with my previously established policy, I did not use any ice loading in combination with wind. Any significant icing of the tower and its guy cables, in addition to wind loading specified for this geographical area, will put the tower and surrounding area in serious danger.

j) In Cases 2(a), 3(a) and 4(a) of my analysis, I assumed that Four Jacks Broadcasting, Inc. will use the Alan Dick Superturnstile Antenna for Channel 2 and I utilized the published design parameters which were adjusted to EIA/TIA Standard 222-E as follows:

Antenna Design Parameters

Height	Weight Including Base Support Frame	Shear	Overturning Moment
104 ft.	17,000 lbs.	8900 lbs.	393,000 ft.-lbs.

k) The tower height, span lengths, guy anchor distances and the antenna loading were taken from the sketch of tower prepared by Gerhold, Cross & Etzel, Professional Land Surveyors, dated 1/20/92.

FINDINGS & EVALUATION

A structural study of the assumed tower geometry, member sizes and the computer analysis of Cases 2(a), 3(a) & 4(a) indicate the following:

1. <u>Under Case 2(a)</u> Tower in its assumed configuration and antenna and transmission line loading as described in the Organization of Analysis Section of this Report, under a 75 mph basic wind velocity and no ice in accordance with EIA/TIA Standard 222-E.

   a) The tower legs are overstressed in 20% of the tower by as much at 64%.

   b) The safety factor of the fourth guy level is 2.4 under wind direction B. This safety factor is below the required by EIA RS 222-E Safety Factor of 2.5.

   c) The column buckling evaluation parameter for the [illegible]

The tower legs are overstressed in approximately 10% of the tower by as much as 64.5%.

4. In Analysis Case 2(a) I assumed that the proposed Channel 2 antenna requires two 3-1/8 inch rigid transmission lines. The Four Jacks Broadcasting, Inc. consultant disputed the need for two 3-1/8 inch rigid transmission lines asserting that one 3-1/8 inch rigid transmission line would be sufficient. Of course, it is easy to see that the only justification for FJB Inc. to plan a low reliability antenna system is to squeeze costs and to support their contention that the tower is safe. However, the FJB's effort to help the tower situation was destined to fail. Below is a comparison of the tower legs overstress levels under Analysis Cases 2(a), 3(a) and 4(a). Under Analysis Case 2(a) it is assumed that there are two 3-1/8" rigid transmission lines and no bundling in the balance of twenty-one other transmission lines. Under Analysis Case 3(a) it is assumed that there is one 3-1/8" rigid transmission line and the balance of the other twenty-one transmission lines are arranged in <u>three</u> bundles. Under Analysis Case 4(a) it is assumed that there is one 3-1/8" rigid transmission line and the balance of the other twenty-one transmission lines are arranged on <u>one</u> bundle.

<u>Analysis Case Comparison</u>

Leg	Case 2(a) Two 3-1/8" Lines No Bundling	Case 3(a) One 3-1/8" Line Three Bundles	Case 4(a) One 3-1/8" Line One Bundle

15	2.3	*	*
*	*	*	*
28	19.6	28.1	29.3
29	54.4	59.0	59.5
30	64.5	64.5	64.5

* Where no stress number is shown or where the numbering of tower sections is not consecutive, it means that there is no overstress in those particular tower sections.

Therefore, the plan to use one 3-1/8 inch rigid transmission line and to bundle all small lines in one impractical bundle did not help the tower situation as far as Four Jacks Broadcasting, Inc. is concerned. Still, 10% of the tower leg sections are overstressed from 29.3 to 64.5%. This, coupled with the fact that no ice loading was considered and allowable stresses were increased by 33%, renders the subject tower unsafe for installing the Channel 2 antenna and on 3-1/8 inch transmission line.

Final Conclusion

It is my engineering opinion that, due to the large overstresses calculated in the tower legs, the subject tower is not adequately designed to support the Channel 2 antenna and its transmission lines as described in the Organization of Analysis Section of this Report. Therefore, I strongly recommend that the subject tower must not be used for the installation of the Channel 2 Antenna.

NOTES

1. The Findings presented in this section are based on the assumed tower geometry, member sizes and properties, guy cable sizes, and the antenna and transmission line loading described herein.
2. The Computer Analysis Results show the safety factors of

the guys and the deflection curve for the tower under Cases 2(a), 3(a) and 4(a). The Computer Analysis Results also list the maximum leg and diagonal loads per tower section.

REPLACEMENT TOWER

The engineering estimate to build a new tower 599 ft. in height on the same site to support the Channel 2 antenna, in accordance with EIA/TIA Standard 222-E, is $300,000.00.

Due to the nature of this Engineering Investigation, I disclaim any liability arising from original design, geometry, material, fabrication and erection deficiencies or the "As Built" condition of the tower. Furthermore, the information and conclusions contained in this Report were determined by application of the current "state-of-the-art" engineering and analysis procedures and formulae, and Vlissides Enterprises, Inc. (Matthew J. Vlissides, P.E.) assumes no obligations to revise any of the information or conclusions contained in this Report in the event that such engineering and analysis procedures and formulae are hereafter modified or revised. In addition, under no circumstances will Vlissides Enterprises, Inc. (Matthew J. Vlissides, P.E.) have any obligations, responsibility or liability whatsoever for or on account of consequential or incidental damages sustained by any person, firm or organization as a result of any information or conclusions contained in this Report.

I declare under penalty of perjury that the foregoing is true and correct to the best of my knowledge.

STATE OF MARYLAND
MATTHEW JOHN VLISSIDES

SECTION B

PART I
COMPUTER INPUT CALCULATIONS

ANALYSIS

CASE 2(a)

/COMTRAN/ GUYED TOWER ANALYSIS
JOB: 599 Ft. Guyed Tower - REVISED HEIGHT - 75 Mph Basic Vel., EIA 222E

3 SIDES
7 SPANS
30 SECTIONS
MISC 5.00% WEIGHT
MISC 2.00% AREA
BASE PIER ELEVATION = 1.00
GUST RESPONSE FACTOR (Gh) = 1.05
BASIC WIND VELOCITY = 75.00
WIND ANGLE = 0
THIS RUN USED VERSION EIA 222-E

SPAN DATA

SPAN	LENGTH (FT)	WIND PRES (PSF)	WEIGHT (KIPS)	WIND LOAD (KIPS)	AVERAGE I (INFT)**2	SYST TYPE	TORS STIFF (KIP-FT)
1	93.500	17.021	5.098	1.988	24.209	N	104.230
2	95.167	22.780	5.155	2.698	24.209	N	104.230
3	95.167	26.441	5.155	3.131	24.209	N	104.230
4	95.167	29.135	5.155	3.450	24.209	N	104.230
5	94.500	31.310	5.119	3.682	24.209	N	104.230
6	95.167	33.160	4.563	3.927	19.550	N	104.230
7	29.832	34.258	1.364	1.272	17.891	N	330.063

SECTION DATA

SEC	PANEL HT (FT)	FACE WDTH (FT)	LENGTH (FT)	SHAPE FACTOR CF	SOL-RATIO e	ROUND S/F Rr
1	5.000	4.000	18.500	2.568	0.209	0.592
2	5.000	4.000	20.000	2.583	0.204	0.591
3	5.000	4.000	20.000	2.583	0.204	0.591
4	5.000	4.000	20.000	2.583	0.204	0.591
5	5.000	4.000	20.000	2.583	0.204	0.591
6	5.000	4.000	20.000	2.583	0.204	0.591
7	5.000	4.000	20.000	2.583	0.204	0.591
8	5.000	4.000	20.000	2.583	0.204	0.591
9	5.000	4.000	20.000	2.583	0.204	0.591
10	5.000	4.000	20.000	2.583	0.204	0.591
11	5.000	4.000	20.000	2.583	0.204	0.591
12	5.000	4.000	20.000	2.583	0.204	0.591
13	5.000	4.000	20.000	2.583	0.204	0.591
14	5.000	4.000	20.000	2.583	0.204	0.591
15	5.000	4.000	20.000	2.583	0.204	0.591
16	5.000	4.000	20.000	2.583	0.204	0.591
17	5.000	4.000	20.000	2.583	0.204	0.591
18	5.000	4.000	20.000	2.583	0.204	0.591
19	5.000	4.000	20.000	2.583	0.204	0.591
20	5.000	4.000	20.000	2.583	0.204	0.591
21	5.000	4.000	20.000	2.583	0.204	0.591
22	5.000	4.000	20.000	2.583	0.204	0.591
23	5.000	4.000	20.000	2.583	0.204	0.591
24	5.000	4.000	20.000	2.583	0.204	0.591
25	5.000	4.000	20.000	2.583	0.204	0.591
26	5.000	4.000	20.000	2.583	0.204	0.591
27	5.000	4.000	20.000	2.583	0.204	0.591
28	5.000	4.000	20.000	2.583	0.204	0.591
29	5.000	4.000	20.000	2.583	0.204	0.591
30	5.000	4.000	20.000	2.583	0.204	0.591

599 Ft. Guyed Tower - REVISED HEIGHT - 75 Mph Basic Vel., EIA 222E

MEMBER DATA: LEGS

SEC	LEG TYPE	DIMENSIONS (IN)	XSECT (SQIN)	I (IN**4)	POUNDS PER FT	SQFT PER FT
1	PIPE	3.500x 0.300	3.016	3.894	10.263	0.173
2	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
3	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
4	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
5	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
6	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
7	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
8	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
9	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
10	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
11	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
12	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
13	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
14	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
15	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
16	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
17	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
18	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
19	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
20	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
21	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
22	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
23	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
24	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
25	PIPE	3.500x 0.300	3.016	3.894	10.263	0.172
26	PIPE	3.500x 0.216	2.228	3.017	7.583	0.172
27	PIPE	3.500x 0.216	2.228	3.017	7.583	0.172
28	PIPE	3.500x 0.216	2.228	3.017	7.583	0.172
29	PIPE	3.500x 0.216	2.228	3.017	7.583	0.172
30	PIPE	3.500x 0.216	2.228	3.017	7.583	0.172

MEMBER DATA: DIAGONALS

SEC	DIAG TYPE	DIMENSIONS (IN)	POUNDS PER FT	SQFT PER FT	PCS
1	ROD	0.625	1.044	0.031	24
2	ROD	0.625	1.044	0.031	24
3	ROD	0.625	1.044	0.031	24
4	ROD	0.625	1.044	0.031	24
5	ROD	0.625	1.044	0.031	24
6	ROD	0.625	1.044	0.031	24
7	ROD	0.625	1.044	0.031	24
8	ROD	0.625	1.044	0.031	24
9	ROD	0.625	1.044	0.031	24
10	ROD	0.625	1.044	0.031	24
11	ROD	0.625	1.044	0.031	24
12	ROD	0.625	1.044	0.031	24
13	ROD	0.625	1.044	0.031	24
14	ROD	0.625	1.044	0.031	24
15	ROD	0.625	1.044	0.031	24
16	ROD	0.625	1.044	0.031	24
17	ROD	0.625	1.044	0.031	24
18	ROD	0.625	1.044	0.031	24
19	ROD	0.625	1.044	0.031	24
20	ROD	0.625	1.044	0.031	24
21	ROD	0.625	1.044	0.031	24
22	ROD	0.625	1.044	0.031	24
23	ROD	0.625	1.044	0.031	24
24	ROD	0.625	1.044	0.031	24
25	ROD	0.625	1.044	0.031	24
26	ROD	0.625	1.044	0.031	24
27	ROD	0.625	1.044	0.031	24
28	ROD	0.625	1.044	0.031	24
29	ROD	0.625	1.044	0.031	24
30	ROD	0.625	1.044	0.031	24

599 Ft. Guyed Tower - REVISED HEIGHT - 75 Mph Basic Vel., EIA 222E

MEMBER DATA: HORIZONTALS

SEC	HORIZ TYPE	DIMENSIONS (IN)	POUNDS PER FT	SQFT PER FT	PCS
1	ROD	1.000	2.673	0.049	15
2	ROD	1.000	2.673	0.049	15
3	ROD	1.000	2.673	0.049	15
4	ROD	1.000	2.673	0.049	15
5	ROD	1.000	2.673	0.049	15
6	ROD	1.000	2.673	0.049	15
7	ROD	1.000	2.673	0.049	15
8	ROD	1.000	2.673	0.049	15
9	ROD	1.000	2.673	0.049	15
10	ROD	1.000	2.673	0.049	15
11	ROD	1.000	2.673	0.049	15
12	ROD	1.000	2.673	0.049	15
13	ROD	1.000	2.673	0.049	15
14	ROD	1.000	2.673	0.049	15
15	ROD	1.000	2.673	0.049	15
16	ROD	1.000	2.673	0.049	15
17	ROD	1.000	2.673	0.049	15
18	ROD	1.000	2.673	0.049	15
19	ROD	1.000	2.673	0.049	15
20	ROD	1.000	2.673	0.049	15
21	ROD	1.000	2.673	0.049	15
22	ROD	1.000	2.673	0.049	15
23	ROD	1.000	2.673	0.049	15
24	ROD	1.000	2.673	0.049	15
25	ROD	1.000	2.673	0.049	15
26	ROD	1.000	2.673	0.049	15
27	ROD	1.000	2.673	0.049	15
28	ROD	1.000	2.673	0.049	15
29	ROD	1.000	2.673	0.049	15
30	ROD	1.000	2.673	0.049	15

MEMBER DATA: REDUNDANT MEMBERS AND ANCILLARY ITEMS

SEC	RDND TYPE	DIMENSIONS (IN)	POUNDS PER FT	SQFT PER FT	PCS	ANCILL WT (K)	ANCILL A (SQFT)
1	NONE		0.000	0.000	0	0.100	0.000
2	NONE		0.000	0.000	0	0.100	0.000
3	NONE		0.000	0.000	0	0.100	0.000
4	NONE		0.000	0.000	0	0.100	0.000
5	NONE		0.000	0.000	0	0.100	0.000
6	NONE		0.000	0.000	0	0.100	0.000
7	NONE		0.000	0.000	0	0.100	0.000
8	NONE		0.000	0.000	0	0.100	0.000
9	NONE		0.000	0.000	0	0.100	0.000
10	NONE		0.000	0.000	0	0.100	0.000
11	NONE		0.000	0.000	0	0.100	0.000
12	NONE		0.000	0.000	0	0.100	0.000
13	NONE		0.000	0.000	0	0.100	0.000
14	NONE		0.000	0.000	0	0.100	0.000
15	NONE		0.000	0.000	0	0.100	0.000
16	NONE		0.000	0.000	0	0.100	0.000
17	NONE		0.000	0.000	0	0.100	0.000
18	NONE		0.000	0.000	0	0.100	0.000
19	NONE		0.000	0.000	0	0.100	0.000
20	NONE		0.000	0.000	0	0.100	0.000
21	NONE		0.000	0.000	0	0.100	0.000
22	NONE		0.000	0.000	0	0.100	0.000
23	NONE		0.000	0.000	0	0.100	0.000
24	NONE		0.000	0.000	0	0.100	0.000
25	NONE		0.000	0.000	0	0.100	0.000
26	NONE		0.000	0.000	0	0.100	0.000
27	NONE		0.000	0.000	0	0.100	0.000
28	NONE		0.000	0.000	0	0.100	0.000
29	NONE		0.000	0.000	0	0.100	0.000
30	NONE		0.000	0.000	0	0.100	0.000